

NORTH CAROLINA Department of Transportation



Hydraulic Planning, Now with Hydroplaning

Stephen Morgan, PE State Hydraulics Engineer

May 17, 2023



NORTH CAROLINA Department of Transportation



NCDOT Hydraulic Planning Report

Brian Radakovic, PE

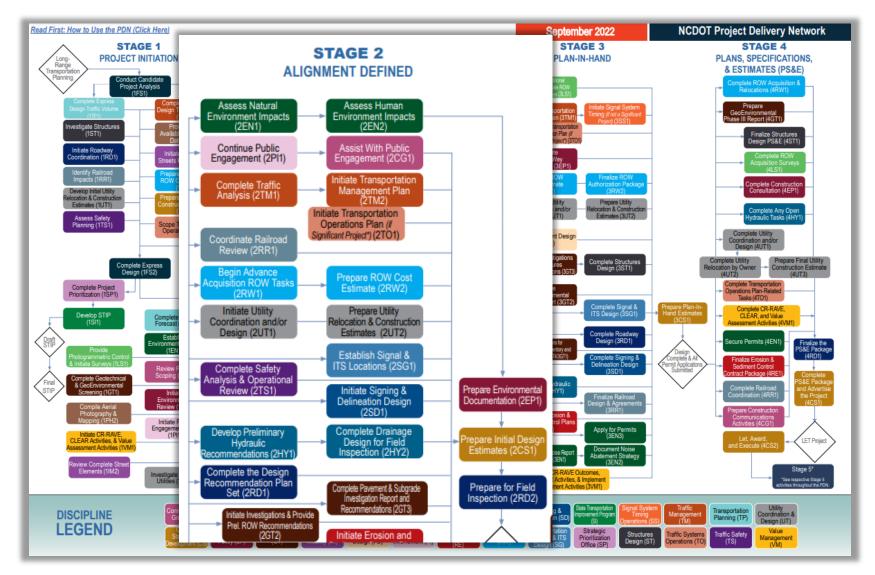
May 17, 2023

- Preliminary hydraulic information to refine line and grade
- Establish the hydrologic performance standards for the project.
- identify permitting requirements;
- Identify risk
- Define avoidance and minimization opportunities;
- Estimate major drainage structure sizes where appropriate.
- Assesses potential Hydroplaning issues (When Necessary)

It's scalable and can cover all project types.

If necessary, preliminary hydraulic modelling is conducted to inform project impacts and revise final alternative.

When Does It Occur?



Hydraulic Planning Report Format

	Cover Page
	General Information
	Roadway Alignment & Site Map *
	Preliminary Major Crossing Table
Hyaraulic Planning Report (submitted as a single PDF,	Site Data *
listed here in order of appearance)	Preliminary Plan & Profile Sketch
	FEMA Site Maps with Sites Labeled
	Photos as Needed
	Preliminary Hydroplaning Assessment (as needed)
	Miscellaneous Data as agreed upon by the QA reviewer

NCDOT - HYDRAULIC PLANNING REPORT
TIP/PROJECT NO. BR-0168 WBS ELEMENT NO. 50603.1.1
PROJECT Replace Bridge 330289 on SR 4000 over US 52 DESCRIPTION:
COPOSED RDWY Unknown - Requested from Rdwy 04/27/2023 TYPICAL:
EXISTING RDWY 5 Lanes on bridge with 2 southbound and 3 northbound lanes. TYPICAL: One northbound lane is left turn lane to ramp onto US 52 northbound.
COUNTY: Forsyth
DIVISION: 9
DESIGNER: Ray D. Lovinggood DATE: 1-May-23
SEAL SEAL SEAL O19775 COVINGINE
PREPARED BY: NCDOT Hydraulics Unit
PROJECT MANAGER: Ray Lovinggood, PE
QA REVIEWED BY: Motthew York, PE

St LEMENT II: 50603.1.1 Forryth NCDOT - HPR EDSIGNT RRM: 45047 DVISION: 9 GENERAL INFORMATION DESIGNT RM: 45047 CELLANEOUS PROJECT INFORMATION Image: Cell Control Contrecont Contrecont Contrecont Control Control Contrecont Control Con		BR-0168		DATE: 5/1/2023
Image: Second		50603.1.1	NCDOT - HPR	DESIGN FIRM: 45047
CELLANEOUS PROJECT INFORMATION ① From Greg Dellocona, County Maintenance Office: The ponded water within the northern loop was created by blocking the outlet pipe. Too much water was flowing in the dirth behind the Applebees, Steek 'n Shoke, and Allegocy Federal Credit Union many years. By blocking the pipe, water heads up in the low sport within the loop and think relief to flow down and in the the channel identified as SA in the NRTR. As far as he know, the water has never gatten into the roads. Mr. Dellacona had no other drainage concerns for this immediate area. No existing stormwater treatment devices No FRM floodplain involvement No current flooding issues No knews related to roadway drainage are in the NRTR. Spread Check on Deck: Assume length = 327' (with opproach slobs) Width = 1/2 x (90) = 45' Cross Slope = 0.02 ft/ft Mannings in' = 0.013				
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EN SHEET COMMITMENTS No Green Sheets yet.			FEMA INVOLVEMENT?	
FEMA INVOLVEMENT?			TES IV NO	
FEMA INVOLVEMENT? TYES FNO	While there passes under	doesn't appear to be FEMA Patterson Ave, the US-52	westbound exit ramp, east and westbound lanes of US-52, and north and s	
FEMA INVOLVEMENT? Image: YES YES While there doesn't appear to be FEMA involvement at this time, there is a FEMA studied stream to the south and southeast of the bridge. Mill Creek posses under Patterson Ave, the US-52 wetboand exit ramp, east and wetboand lanes of US-52, and north and southboanad lanes of SR-4000. By keeping the project limits contained near the existing bridge, the stream should not be a factor.				
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PROJECT NUMBER: U-5307 WBS ELEMENT #: COUNTY: Wake

5

DIVISION:

PRELIMINARY HYDRAULIC RECOMMENDATIONS FOR MAJOR CROSSINGS

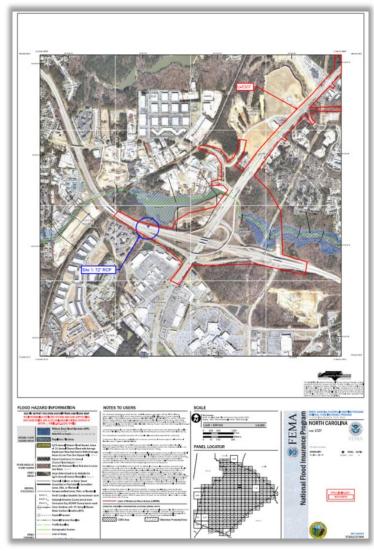
DATE: DESIGN FIRM: DESIGNED BY: John Doe, PE REVIEWED BY:

											MINIMUM RECOMMENDED		
	ALTERNATIVE ID					STREAM/		FEMA STUDY	DRAINAGE	EXISTING STRUCTURE	STRUCTURE		
SITE NUMBER	(2)	ROUTE	STATION	LAT	LONG	WETLAND ID	STREAM NAME	ТҮРЕ	AREA (Mi^2)	Number, Size, Structure Type	Number, Size, Structure Type	Notes	
1	Northern Wake Expy	I-540	-Y1- Sta. 21+43	35.87664	-78.58303	N/A	Unnamed Tributary	N/A	0.23	72" RCP	Retain existing 72" RCP		
2	Northern Wake Expy	I-540	-Y1- Sta. 62+86	35.87383	-78.56939	N/A	Unnamed Tributary	N/A	1.27	2 @ 8' x 6' RCBC	Retain 2 @ 8' x 6' RCBC and add a supplemental 72" WSP by Trenchless Installation.	Infiltration was observed along the joints on the sidewalls of the culvert and	
3	Gresham Lake Rd.	SR-2023	-Y9C- Sta. 52+00	35.87597	-78.56850	27-25-(2)	Perry Creek	Detailed Study	5.33	N/A	Propose 300' reinforced concrete bridge on girders		
4	Capital Boulevard	US-1	-L- Sta. 60+35	35.87935	-78.57339	27-25-(2)	Perry Creek	Detailed Study	3.74	3 @ 8' x 8' RCBC	Retain and extend 3 @ 8' x 8' RCBC approx. 40' D/S and add a supplemental 84" WSP by Trenchless Installation.		
5	Capital Boulevard	US-1	-L- Sta. 101+92	35.88815	-78.56454	27-25-3-(2)	Unnamed Tributary Near Neuse	N/A	3.14	2 @ 8' x 8' RCBC	Retain 2 @ 8' x 8' RCBC and add a supplemental 84" WSP by Trenchless Installation.		
6	N/A	N/A	-Y10- 52+32	35.88733	-78.56097	27-25-3-(2)	Unnamed Tributary Near Neuse	N/A	3.49	N/A	3 @ 10' x 10' RCBC		
7	N/A	N/A	-Y10- 65+86	35.88973	-78.55756	N/A	Unnamed Tributary	N/A	0.1	N/A	1 @ 6' x 7' RCBC		
8	Capital Boulevard	US-1	-L- 185+00	35.90891	-78.55459	27-(22.5)	Neuse River	Detailed Study	792	248' Dual Bridges w/ RC Floor/Prestressed Conc. Girders	3 proposed 280' reinforced concrete bridges on girders		
9	N/A	N/A	-Y18A- 62+36	35.94112	-78.53702	N/A	Unnamed Tributary	N/A	0.21	N/A	1 @ 7' x 8' RCBC		
10	Capital Boulevard	US-1	-L- Sta. 388+50	35.96153	-78.54240	27-21-(1.5)	Richland Creek	Detailed Study	10.4	4 @ 10' x 11' RCBC	Retain and extend 4 @ 10' x 11' RCBC approx. 10' U/S and 10' D/S.		
11	N/A	N/A	-Y10F- Sta. 10+81	35.88767	-78.56358	27-25-3-(2)	Unnamed Tributary Near Neuse	N/A	3.15	N/A	3 @ 9' x 8' RCBC		

WBS ELEMENT #:		1				-	DESIGN FIRM	
COUNTY: DIVISION:	Wake 5	Ĺ	T)	SITE 1		-	DESIGNED BY REVIEWED BY	: John Doe, PE
						-		
ISTING STRUCTUR								
Str. #:	920867		5.87664	Longitude: -78.5		Stream:	UT to Pe	rry Creek
Structure Type:	RCP		Yr Built:	Unknown	Skew: 95	River Basin:	N	PUSe
Exist. Str. Info: S	ingle Barrel 72" RC	CP w/ Beveled Ed	dge Head	wall & Endwall				
Bed to Crown (ft):	40	Clear Roady			ter Depth (ft):	0.3	OAL (ft)	
Existing Structure C	outlet perched 30" f	rom inv. to stream	m bottom	. GIS database note tion as fair with crac	is NCDOT Non-Na	tional Bridge In ne in the last 3	spection Standa	ard (Non-NBIS)
110101	the constraint table	ioucon nata tre p	npe contai		no along the now i		pipe joints clos	and to the outlet.
ADT:	97,800	Year ADT: 20	020	Scour Code(Item	113): N/A	Prior Survey:	Yes, 2	/2/2021
Flooding late 1.				that Site 1 experier		t has not overto		
riooung mo 1.1	CDOT DIVISION DE	snoge mannenan	106 310100	and one respond	iooa nigit water ou		pped in the pas	
L								
Flooding info 2: N	lo secondary sourc	e for flooding info	ormation	available.				
ANNEL INFORMA	TION							
S Channel Condition:		12' top width 1' tr	o 2º denth	2-1 side slopes I t	& Rt 4" to 6" flow	depth_apod_co	ndition no sign	s of erosion
Champer Containion: 4	0000 Width, 0 10 1	12 top widen, 1 to	o z dopin	, 2.1 alue alupea Lt.	d RL, 4 10 0 104	depail, good oc	numon, no sigi	a of erosion.
L								
5 Channel Condition: A								to 12' top width,
				24' long (scour hole " flow depth, good c				to 12' top width,
								to 12' top width,
1	' to 2' depth, 2:1 sid							to 12' top width,
1	' to 2' depth, 2:1 sid	de slopes Lt. & R		* flow depth, good c			risham Lake.	' to 12' top width, mry Creek
1 STREAM FEATURE	' to 2' depth, 2:1 sid	de slopes Lt. & R	tt., 4" to 6	* flow depth, good c	endition, before co	nfluence with G	risham Lake. UT to Pe	
1 STREAM FEATURE Str. #: U/S Feature:	' to 2' depth, 2:1 sid	de slopes Lt. & R Latitude: 35 Yr Built: Unk	8t., 4" to 6	* flow depth, good c Longitude: -78.5 Skew: N//	endition, before co	Stream:	risham Lake. UT to Pe	nrry Creek
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STREAM FEATURE Str. # [N U/S Feature Info U/S Feature Info Bed to Crown (ft) WNSTREAM FEAT Str. # [N D/S Feature Info D/S Feature Info Bed to Crown (ft) Drainage Area Future Land Future Indo	VA VA Vetention Pond VA VA VA Vetention Pond Beh V/A	Lestitudes 35 Yr Built- Unk aind dollar tree pa Cleor Roody Lotitude: 35 Yr Built- N Cleor Roody Cleor Roody Lotitude: 35 Yr Built- N Cleor Roody L Cleor Roody Cleor Roody L Cleor Rood		** flow depth, good of Longitude: -78.5 Skew: N// Outlet structure is a N/A Wa' Longitude: -78.5 Skew: N// Jungitude: -78.5 Skew: N// Muence with Gresha N/A N/A Wa' Area Source: USC loogic Region: Reg ionatic warspresk, Futu a metal portion of C a: metal portion of C a metal portion of C	endition, before co 8345 Gooole Maps A 1 24* RCP. ter Depth (tr) N/A 8110 Gooole Maps A m Lake. ter Depth (tr) N/A ter Depth (tr) N/A SS Stream Stats ion 1 – Piedmont ndustrial, and Vaca piormmunity Mixed U	Stream [Route] Stream [Route] Stream [Route]]]] Fot] Stream Gage]] Fot stream Gage]	UT to Pr Perry Creek/i Perry Creek/i re % Impervious # (if applicoble) h falls under oc rgoly falling winder vious #	Irry Creek I/A Gresham Lake I/A 41% N/A Inin Regional rea delineated id not overtepping.

PROJECT NUMBER: U-5307			DATE:
WBS ELEMENT #:	NCDOT	- нрк	DESIGN FIRM:
COUNTY: Wake		1	DESIGNED BY: John Doe, PE
DIVISION: 5		: 1	REVIEWED BY:
FEMA			
Type of FIS:	N/A Date of FIS:	N/A Regulato	ry Floodway Width: N/A (Noted in FIS)
River Station:	N/A RDWY OT @ Q100?:	NO Panel #:	1727 Panel Date: 5/2/2006
Damage Pote	ntial?: Low Could pro	oposed structure significantly	increase damages?: Don't Know
*Buildings in Flood	plain?: Yes Explanation of Increased [Damages: Potenti	ial increases due to roadway widening
List Buildings in Apartment he Flood Plain w/ Location & Floor Elev.:	ousing 250 ft southeast upstream from inlet. May o	or may not be within flood	plain. No floor elevation at this time.
CLOMR/SFC Est	imate: N/A		
HIGHWAY & BRIDGE/CULVERT	RELATED EVALUATIONS		
Are there any outside features t No	hat might affect stage, discharge or frequency?:		
ONSITE DETOUR INFORMATION	N		
Structure Type: N/A			
Detour Str. Info: N/A			
DESIGN CONCERNS			
None			
Structure Type:	RCP		e note if extending/widening/retaining
Retain Existing	72" RCP; Div. 5 noted it will require minor preservatio	in work.	
Proposed Structure and any grade			

What is the Hydraulic Planning Report?





Site 1: Looking Downstream from Top of Outfall



Site 1: Downstream Channel

			PRECON	NSTRUCTION DAS	HBOARD		- 1
				BR-0168			
			Project Ma	anager : Hernandez	Jennifer		- 1
	All Projects	♦ Division	09 ≑	BR-0168		× × Add Project	
et our	regulatory requirements. Whi	cisions are made throughout the le all projects have some level	e project lifecycle. NCDOT of A&M measures, the Merg	ger Process may include more	pacts to the natural and human detailed A&M discussions with re	esource agencies.	- I
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et our s list p e guida	regulatory requirements. Whi rovides a means of tracking A nce here. New Item	cisions are made throughout the le all projects have some level &M measures that can support Description	e project lifecycle. NCDOT of A&M measures, the Merg t the 404 permit application Create Date	seeks to avoid and minimize im ger Process may include more process, in particular. For more Change Date	pacts to the natural and human detailed A&M discussions with re e information on A&M contact EA Originator	esource agencies. AU or EPU staff, or Modified By	

Special Project Commitment (aka GreenSheets)

FEMA Floodplain (Hydraulics)

Title: Construction in FEMA Floodplain

Project Commitment Standard Language:

This project involves construction activities on or adjacent to FEMA-regulated stream(s). Therefore, the Division shall: (1) construct all vertical and horizontal elements within the floodplain as designed; and (2) consult with the Hydraulics Unit of any planned deviation of these elements within the floodplain prior to commencing any such changes; and (3) submit sealed as-built construction plans to the Hydraulics Unit upon completion of project construction. The Hydraulics Unit will then verify either: (1) the drainage structure(s) and roadway embankment located within the 100-year floodplain were built as shown in the construction plans, both horizontally and vertically; or (2) any changes made to the plans were reviewed and approved to meet FEMA SFHA compliance; or (3) appropriate mitigation measures will be achieved prior to project close-out.

Commitment Reason: STIP Project [TIP #] crosses FEMA-regulated stream [Stream name].

Stakeholder: Highway Floodplain Program

Stakeholder Email(s):

Phase: Post-Construction

Commitment Validation

Discipline Responsible: Hydraulics Unit

Person Assigned to Validate: [Hydraulics Unit Representative]

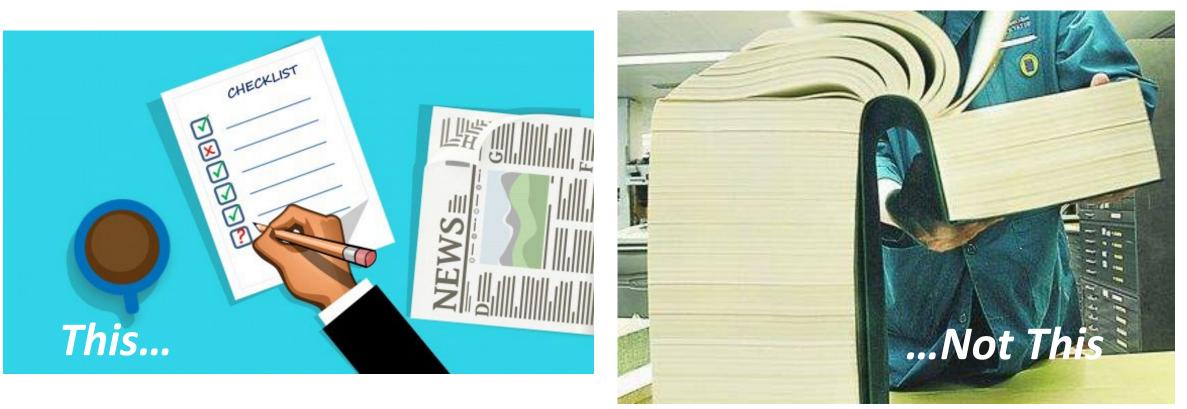
Validation Due Date: [Two weeks from entry]

https://connect.ncdot.gov/resources/Environmental/EPU/Policy/Documents/Special_Project_Commitment_Guidance.pdf

Special Project Commitment Guidance

Version 1.0

April 2023



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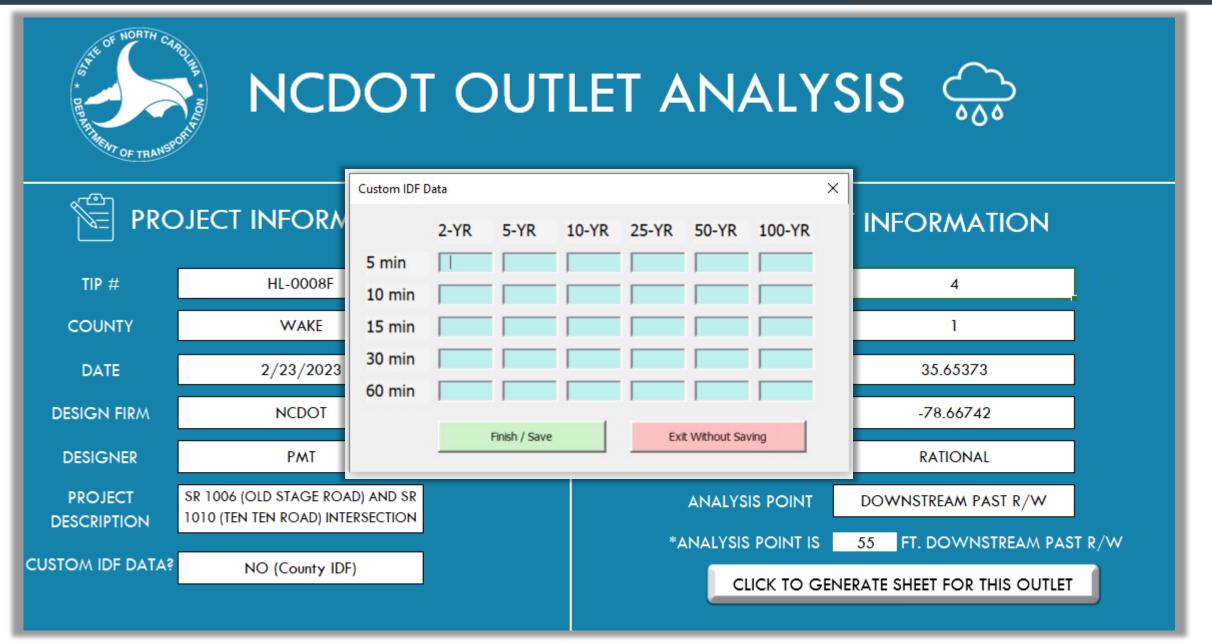
NORTH CAROLINA Department of Transportation



NCDOT Outlet Analysis Tool

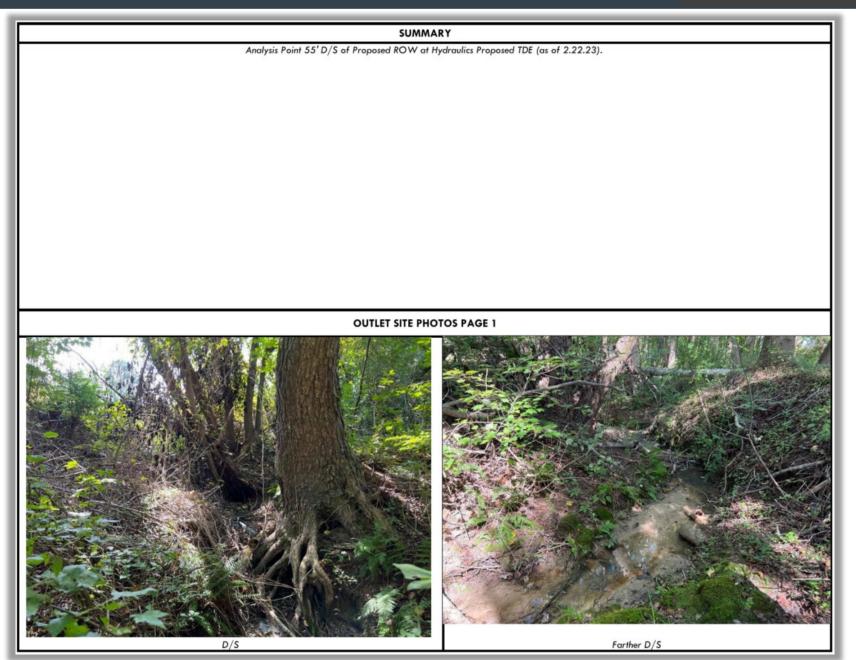
Matthew York, PE

May 17, 2023



TIP Project:	HL-0008F		Date:		2/22/2023
nter Pre Sub DA & C value	WAKE	DUTLE	Enter Post Sub DA & C value		0
Drainage Area Impervious within R/W / Easement / NCDOT Contribution (Acres)		AKEN 5: Latitud POST-	Drainage Area Impervious within R/W / Easement / NCDOT Contribution (Acres)	2.89	C Value Impervious 0.9
Drainage Area Impervious Outside R/W Easement (Acres)	1.95	o Enter I	Drainage Area Impervious Outside R/W / Easement (Acres)	1.95	
Drainage Area Grass within R/W / Easement / NCDOT Contribution (Acres)	0.00 C Value Grass 0.3	.C (min (in/hr) (in/hr)	Drainage Area Grass within R/W / Easement / NCDOT Contribution (Acres)	0.00	C Value Grass 0.3
Drainage Area Grass Outside R/W / Easement (Acres)	25.95	(in/hr) (in/hr) (in/hr)	Drainage Area Grass Outside R/W / Easement (Acres)	24.8	
Drainage Area Woods within R/W / Easement / NCDOT Contribution (Acres)	0.00 C Value Woods	(in/hr) p in R/W	Drainage Area Woods within R/W / Easement / NCDOT Contribution (Acres)	0.00	C Value Woods 0.2
Drainage Area Woods Outside R/W / Easement / NCDOT Contribution (Acres)	0.00	DA _{total} reu	Drainage Area Woods Outside R/W / Easement / NCDOT Contribution (Acres)	0.00	0.2
Drainage Area Other #1 within R/W / Easement / NCDOT Contribution (Acres)	0.00 C Value Other #1	201 CULATE	Drainage Area Other #1 within R/W / Easement / NCDOT Contribution (Acres)	0.00	C Value Other #1
Drainage Area Other #1 Outside R/W / Easement / NCDOT Contribution (Acres)	0.00	ist. D/S	Drainage Area Other #1 Outside R/W / Easement / NCDOT Contribution (Acres)	0.00	U
Drainage Area Other #2 within R/W / Easement / NCDOT Contribution (Acres)	0.00 C Value Other #2	NSTRU	Drainage Area Other #2 within R/W / Easement / NCDOT Contribution (Acres)	0.00	C Value Other #2 0
Drainage Area Other #2 within R/W / Easement / NCDOT Contribution (Acres)	0.00	NSTRU	Drainage Area Other #2 within R/W / Easement / NCDOT Contribution (Acres)	0.00	
Drainage Area Other #3 within R/W / Easement / NCDOT Contribution (Acres)	0.00 C Value Other #3	Π	Drainage Area Other #3 within R/W / Easement / NCDOT Contribution (Acres)	0.00	C Value Other #3
Orainage Area Other #3 within R/W / Easement / NCDOT Contribution (Acres)	0.00		Drainage Area Other #3 within R/W / Easement / NCDOT Contribution (Acres)	0.00	U
Compute & Save	Exit Without Saving		Compute & Save		Exit Without Saving

TIP Project: County: Description:	HL-0008F WAKE SR 1006 (OLD STA (TEN TEN ROAD)		ID SR 1010		PSH 4 OL		- 26+89 LT	ST P/W	Dat Design Firr Designed B Reviewed B	m:	O PMT		
	IMDDOV/EMENITC	JID Number:	OA-HL-000				35.65373	131 K/ W	Longitud		8.66742	Google Maps	
PRE-0	CONSTRUCTION (R	_				-	ONSTRUCTION	RATIONAL	-			NGE	
		Q ₂ =	40.3	cfs				Q ₂ =	44.8	cfs	Q ₂ =	11.3%	
		V ₂ =	1.9	ft./s				V ₂ =	2.0	ft./s	V ₂ =	2.7%	
C _{composite} =	0.36	D ₂ =	3.3	ft.	Cron	mposite =	0.40	D ₂ =	3.5	ft.	D ₂ =	5.4%	
T.O.C (min.)		Q ₅ =	46.6	cfs		(min.)	15.0	Q ₅ =	51.9	cfs	Q ₅ =	11.3%	
I ₂ (in/hr) =	3.80	V ₅ =	2.0	ft./s		n/hr) =	3.80	V ₅ =	2.0	ft./s	V ₅ =	2.7%	
I ₅ (in/hr) =	4.40	D ₅ =	3.6	ft.	I₅ (ii	n/hr) =	4.40	D ₅ =	3.8	ft.	D ₅ =	5.3%	
I ₁₀ (in/hr) =	4.89	Q ₁₀ =	51.8	cfs	I ₁₀ (ii	n/hr) =	4.89	Q ₁₀ =	57.7	cfs	Q ₁₀ =	11.3%	
I ₂₅ (in/hr) =	5.39	V ₁₀ =	2.0	ft./s	I ₂₅ (ii	n/hr) =	5.39	V ₁₀ =	2.1	ft./s	V ₁₀ =	2.7%	
I ₅₀ (in/hr) =	5.76	D ₁₀ =	3.8	ft.	I ₅₀ (ii	n/hr) =	5.76	D ₁₀ =	4.0	ft.	D ₁₀ =	5.3%	
DA imp in R/W =		Q ₂₅ =	57.1	cfs	DA imp i	in R/W =	2.89 ac.	Q ₂₅ =	63.6	cfs	Q ₂₅ =	11.3%	
DA _{total} =	29.08 ac.	V ₂₅ =	2.1	ft./s	D	A _{total} =	29.64 ac.	V ₂₅ =	2.1	ft./s	V ₂₅ =	2.7%	
		D ₂₅ =	3.9	ft.				D ₂₅ =	4.1	ft.	D ₂₅ =	5.2%	
% Imp. Area		Q ₅₀ =	61.1	cfs	% Imp. Areo			Q ₅₀ =	67.9	cfs	Q ₅₀ =	11.3%	
(NCDOT Contribution)	4.06%	V ₅₀ =	2.1	ft./s	(NCDOT Contrib	bution)	9.75%	V ₅₀ =	2.2	ft./s	V ₅₀ =	2.7%	
		D ₅₀ =	4.1	ft.				D ₅₀ =	4.3	ft.	D ₅₀ =	5.2%	
											DA _{total} =	1.9%	
Railroad / 100-YR	No	Existing Erosio	n / Potentia	Both	Post Const.	D/S T	S a ti				DA imp R/W =	1.71 ac.	
Design?	140	Future Er	rosion?	Both	Channel line	r type	Soil				% imp Area =	5.7%	
SOIL TYPE	:	Sandy loam			PRE-CON	STRUCTI	ON OUTLET GE	OMETRY		Tr	apezoidal		
V _{10 Permissible}	2.5 ft./s				POST-CONS	TRUCTIO	N OUTLET GEO	OMETRY	G	eometry Sa	me as Pre-Construc	tion	
τ _{10 Permissible}	N/A	0	Case 3A		← Case Type					,			
PRE	-CONSTRUCTION	DUTLET GEOME	TRY					POST-C	ONSTRUCTIO	N OUTLET	GEOMETRY		
Natural Grour	nd —	Natur	ral Ground	I									
ŀ	н:∨ В —	H:V					Same Geome	etry as Pre-C	onstruction (S	ee Image a	nd Variables to	the Left)	
Depth (ft) =	6		H:V (Lt) =	1.0									
Slope (ft/ft) =	0.004		H:V (Rt) =										
BANKS	in, etc.). VEGETATED.	ENOSION ALONG	2 0/HR3. 31										
Slope (ft/ft) = Manning's N = Notes (lining, condition	0.004		H:V (Rt) = Width (ft.)	= 1.0 = 3									





How the Tool Accounts for 15A NCAC 04B .0109

		 (a) Persons shall provide a des year storm runoff in the receiving greater of: 	RMWATER OUTLET PROTECTION sign for the land-disturbing activity so that the post- ng stormwater conveyance to, and including, the disc stablished by the table in Paragraph (d) of this Rule;	harge point, does not exceed the
Brings Cor Brings Cor	Material		um Permissible et and Meters Per Second* M.P.S.	rmwater conveyance prior to be met, then the receiving constructed to withstand the n percent. mative measures to control s by designing measures to
Documenta	Fine Sand (noncolloidal)	2.5	.8	ted or roughened swales and
Formalizes	Sandy Loam (noncolloidal) Silt Loam (noncolloidal) Ordinary Firm Loam	2.5 3.0 3.5	.8 .9 1.1	 v velocities to the discharge improving cross sections or n accelerated erosion in the
Eliminates	Fine Gravel	5.0	1.5	n accelerated crosson in the
Assists Sto	Stiff Clay (very colloidal) Graded, Loam to Cobbles (noncolloidal)	5.0 5.0	1.5 1.5	issible ters Per Second*
Audits	Graded, Silt to Cobbles (colloidal) Alluvial Silts (noncolloidal)	5.5 3.5	1.7 1.1	
The Guidel	Alluvial Silts (colloidal)	5.0	1.5	
	Coarse Gravel (noncolloidal) Cobbles and Shingles	6.0 5.5	1.8 1.7	
between m	Shales and Hard Pans	6.0	1.8	
and comm	iny available ooli	Shales and Hard Pans	6.0 1.8	_
Information		channels, and by 0.8 for highly	ly allowable velocity by 0.95 for slightly sinuous, sinuous channels. Source: Adapted from recommend Society of Civil Engineers, 1926, for channels with st	lations by Special Committee on

History Note: Authority G.S. 113A-54(b); 113A-54(c); Eff. February 1, 1976; Amended Eff. February 1, 1992; May 1, 1990; November 1, 1984; July 1, 1978; Readopted Eff. April 1, 2020.

How the Tool Accounts for 15A NCAC 04B .0109

Outlet Analysis Case	Existing Erosion	Pre vs Post Velocity	Post Velocity vs. Permissible Soil Velocity	Velocity Increase 10% Threshold	Rule 15A NCAC 04B .0109 Satisfied?	Additional Requirements / Description
Case 1	Not Present	Decrease or No Change	V10 Post < Permissible for that Soil	N/A Compliance Already Met	YES	Mark "None" in the "Existing / Potential Erosion?" drop down cell
Case 1A		as Case 1	except with existing erosion present	Same	YES	Even though there is a decresae in velocity and it is not erosive, for unknown reasons there is existing erosion present. Mark "Existing" in the "Existing / Potential Erosion" Cell drop down.
Case 2	Not Present	Decrease or	V10 Pact > Parmissible for that Sail	N/A Velocity	VEC	Even though there is a decrease in velocity the flows are still erosive, Mark "Potential" in the "Evisting / Potential Erosion" Coll drop down
Case 2A		No	Existing Erosion / Pote Future Erosion?	ential Bot	h Post Const. Channel line	r type
Case 3	N					y it is not erosive. ell drop down
Case 3A	<u> </u>		Sandy loam		PRE-CON	STRUCTION OUTLET GEOMETRY ity is not erosive, eason. Mark "Both" down
Case 4	2.5	ft./s	Case 3A	4		STRUCTION OUTLET GEOMETRY
Case 4A			פגנפער שונוז פגוזנוווע פוטזוטוו עופצפוונ		←Case Type(ng Erosion and V10
					120	Erosion" Cell drop down
Case 5	Not Present	Increase	V10 Post > Permissible for that Soil	V increase > 10%	NO	Design receiving channel to withstand Vpost anywhere it exceeds Vpre by 10%. Create another outlet analysis tab at that point downstream to show Vincrease < 10%. Coordination with NCDOT may be needed for potential
Case 5A		as Case 5	except with existing erosion present	Same	NO	design solutions or design exceptions in this case due to easement constraints or environmental permitting constraints.
drop dowi	ase of a PROPO. n". Other analysi	SED non-soil line is points further o	er to absolve a Case 5 or 5A, Rule15A N downstream for where Vincrease becol	ICAC 04B .0109 is co mes < 10% are still	nsidered satisifed at this loc necessary.	ration. Mark only "Existing" or "None" in the "Existing / Potential Erosion Cell
Note 2						

² In the case of an EXISTING non-soil liner to remain post-construction, Rule 15A NCAC 04B .0109 does not provide guidance. Therefore, engineering judgement on the existing/potential erosion issues shall be used and discussed in the summary section of the outlet analysis. A combination of the 10% velocity increase threshold, the suggested max velocities, and suggested max shear stresses shall applied to make a design decision in these situations.

What's Next?

0.9	ACRES 1.18 1.95	POST-CONS LAND USE IMPERVIOUS AREA WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES) IMPERVIOUS AREA OUTSIDE RIGHT OF WAY / EASEMENT (ACRES)	C-VALUE	ACRES 2.89	CH.	ANGE Δ SUB AREA (%) 144.9%
0.9 -	1.18	IMPERVIOUS AREA WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES) IMPERVIOUS AREA OUTSIDE RIGHT			(AC.)	(%)
		OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)	0.9	2.89	1.71	144.9%
	1.95		0.7			
				1.95	0.00	0.0%
0.2 L	-	GRASS AREA WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)	0.3	-		
0.5	25.95	GRASS AREA OUTSIDE RIGHT OF WAY / EASEMENT (ACRES)	0.5	24.80	-1.15	-4.4%
0.3 - 0.2 -	-	WOODS AREA WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)	0.2	-		-
		WOODS AREA OUTSIDE RIGHT OF WAY / EASEMENT (ACRES)		-		-
	-	OTHER LAND USE #1 WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)		-		-
-	-	OTHER LAND USE #1 OUTSIDE RIGHT OF WAY / EASEMENT (ACRES)	-	-		-
0.3 0.2		OTHER LAND USE #1 WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)		-		-
	-	OTHER LAND USE #1 OUTSIDE RIGHT OF WAY / EASEMENT (ACRES)		-		
	-	OTHER LAND USE #1 WITHIN RIGHT OF WAY / EASEMENT / NCDOT CONTRIBUTION (ACRES)				
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Land Use Breakdown



What's Next?

Outlet Analysis Summary

TIP Project: HL-0008F County: Version 1.1							SUMMARY TABLE							Date: 2/23/2023 Design Firm: NCDOT						
RATIONAL METHOD SUMMARY TABLE																				
OUTLET INFORMATI	ON	PRE	-CONST	RUCTIC	N SUM	MARY	,	POS	T-CON	STRUCT	ION SU	MMAR	Y				OVERALL SU	MMARY		
OUTLET	MAPS LINK	DRAINAGE AREA (AC.)		% I.A. TOTAL		D10 (FT)	V10 (FT/S)	DRAINAG E AREA (AC.)	C VAL.	% I.A. TOTAL	Q10 (CFS)	D10 (FT)	V10 (FT/S)	۵ Q10	۵ D10	∆ v10	∆ TOTAL AREA (AC.)	∆ IMP AREA (AC.)	۵ ۱.а.	CASE TYPE
PSH 4 OUTLET 1	<u>Click</u>	29.08	0.36	10.8%	51.8	3.8	2.0	29.64	0.40	16.3%	57.7	4.0	2.1	11.3%	5.3%	2.7%	0.56	1.71	5.6%	Case 3A





NORTH CAROLINA Department of Transportation



Stormwater Permit Compliance Planning – Attaining the Maximum Extent Practicable Treatment Standard

Andrew McDaniel, PE NCDOT Preconstruction Workshop May 17, 2023

Agenda

- Maximum Extent Practicable (MEP) Stormwater
 Treatment Explained
- Tools For Success

Some Context

- How many of you do drainage design work for municipal clients?
- Depending on where you are in the state there can be multiple stormwater permits to apply for and obtain
 - Municipal stormwater permit
 - State stormwater permit
- These permits have prescriptive design requirements, which by definition, meet the MEP standard

NCDOT Projects Do Not Apply For Stormwater Permits

- Why?
- Because your project receives automatic coverage under NCDOT's statewide NPDES stormwater permit
- NC General Statutes and NC Administrative Code have consolidated stormwater requirements for NCDOT projects largely under the NPDES permit
 - No municipal stormwater permit required
 - No state stormwater permit required

NPDES Stormwater Permit

- Automatic coverage for:
 - Roadway projects
 - Non-roadway facilities
 - Rail projects
 - Ferry terminal projects
 - NC Global TransPark projects
 - and others

STATE OF NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL QUALITY DIVISION OF ENERGY, MINERAL, AND LAND RESOURCES

PERMIT NO. NCS000250

TO DISCHARGE STORMWATER AND BORROW PIT WASTEWATER UNDER THE

NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM

In compliance with the provisions of North Carolina General Statute 143-215.1, other lawful standards and regulations promulgated and adopted by the North Carolina Environmental Management Commission, and the Federal Water Pollution Control Act, as amended,

North Carolina Department of Transportation

is hereby authorized to discharge stormwater from the North Carolina Department of Transportation (NCDOT) Transportation Separate Storm Sever System (TS4), borrow pit wastewater, industrial and construction activities located statewide in accordance with the discharge limitations, monitoring requirements, and other conditions set forth in Parts 1, 2, 3, 4, 5, 6, 7, and 8 hereof.

This permit shall become effective May 1, 2022.

This permit and the authorization to discharge shall expire at midnight on April 30, 2027.

Signed this day April 26, 2022.

Danny Smith, Stopfiwater Program Supervisor Division of Energy, Mineral, and Land Resources By the Authority of the Environmental Management Commission

This sounds great!

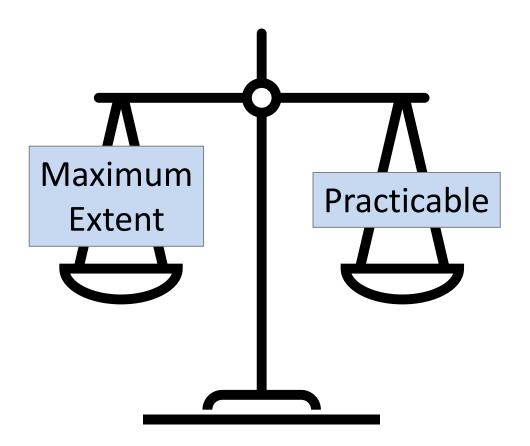
- No stormwater permit applications can speed up project delivery
- Reduces uncertainty and project risks



Or Does It?

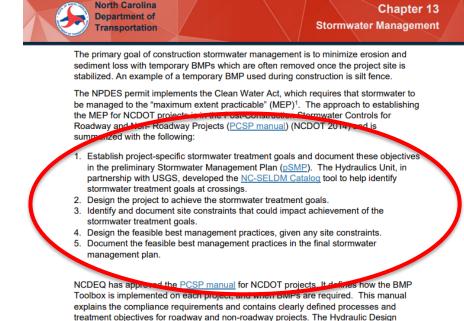
- By design the permit requires NCDOT to develop and implement a Post-Construction Stormwater Program (PCSP)
- Also, by design the permit does not mandate a prescriptive stormwater treatment standard for the PCSP other than the MEP standard
- So, it can be confusing as to what MEP means for NCDOT projects
- Decentralized project delivery complicates the matter further

Finding The Right Balance...



The Good News

 Page 13-6 of the NCDOT Drainage Design Guidelines outlines a straightforward 5 step process to demonstrate attainment of the MEP standard.



Step 1

- Establish project-specific stormwater treatment goals at each crossing and document them in the preliminary Stormwater Management Plan (pSMP).
- A goal is one of the following:
 - Use of a BMP Toolbox structural practice
 - Use of non-structural Minimum Measure practices
 - Direct discharge

NC-SELDM Catalog Tool

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	Upstream basin c	haracteristics (from StreamStats - see the 'Detailed Instructions' worksheet).		
	Parameter Code	Parameter Description	Value	Unit
	CSL10 85fm	Change in elevation between points 10 and 85 percent of length along main channel to basin divide divided by length between points.	vuiue	feet per mile
	DRNAREA	Area that drains to a point on a stream.		square miles
	LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset.		percent
	PCTREG1	Percentage of drainage area located in Region 1 - Piedmont / Ridge and Valley.		percent
	PCTREG2	Percentage of drainage area located in Region 2 - Blue Ridge.		percent
	PCTREG3	Percentage of drainage area located in Region 3 - Sandhills.		percent
	PCTREG4	Percentage of drainage area located in Region 4 - Coastal Plains.		percent
		5 5 5		
	Highway drainage	e area characteristics (from preliminary design plans - see the 'Detailed Instructions' worksheet).		
	HIGHWAY DRAINAGE AF			
	Parameter Name	Parameter Description	Value	Unit
	Drainage area size	Highway and bridge deck area that drains to the highway-stream crossing of interest.		acres
	Avg. highway slope	Average highway slope draining to the stream (elevation change btw. highest and lowest points divided by the length of the flow-path).		percent
	Avg. drainage length	Average length of the flow-paths between the highest and lowest elevations on the highway that drain to the stream.		feet
	Output (raw outp	ut below, details in the 'Report' worksheet).		
	OUTPUT BASED ON INPU			
	Output	Output Explanation		
	#N/A	Error - check input parameters		
		No result returned - check for errors in the input parameter cells.		
		Valid result returned based on input parameters.		

33

Training: NC-SELDM Catalog

NC SELDM video tutorial #2 – Determination of NC SELDM Highway Catchment







U.S. Department of Transportation Federal Highway Administration



34

Preliminary Stormwater Management Plan (pSMP)

- Document Stormwater Treatment Goals in the pSMP
 - Excel form (SMPv3.01) Dec. 2021
 - General Project Information
 - Waterbody Information

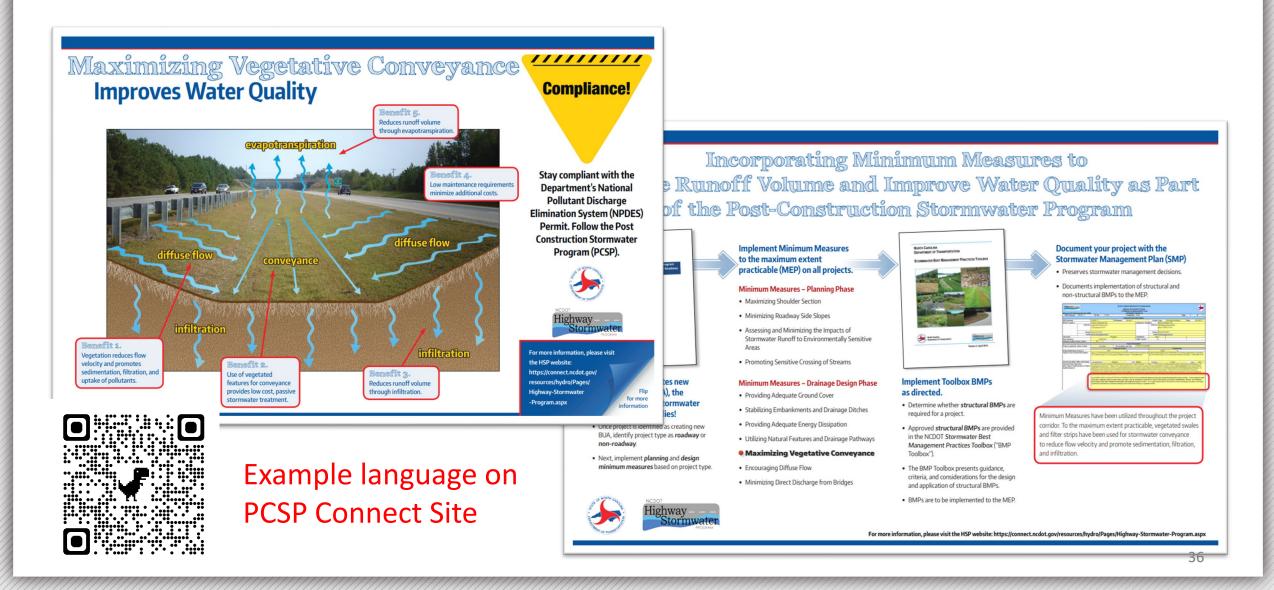
https://connect.ncdot.gov/resources/hydro/Pages/HS PProductPages.aspx?PROD=SMP



- Save a pdf to the Preconstruction site.
 - For projects not on the Preconstruction Site submit via email to NCDOT_Hydraulics_SMP@ncdot.gov

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Minimum Measures Infographic



Step 2

• Attempt to design the project to achieve the stormwater treatment goals.

Step 3

 If necessary, identify and document site constraints that impact achievement of the stormwater treatment goals.



Step 4

- Design the feasible BMP, given the site constraints.
- Keep in mind that the stormwater treatment goal provided by the NC-SELDM Catalog is for water <u>quality</u> treatment.
- Your feasible BMP choice should also factor in water <u>quantity</u> management (outlet analysis).

Step 5

• Document the feasible stormwater control measures in the final stormwater management plan.

	ater		Highway STORMWA	Department of Transpo Stormwater Program FER MANAGEMENT P DE BEDOT PROJECTS					
WBS Element	eaned August 2021	TIP/Proj No:		ountyfies				Page 1	of
				Project Information					
WBS Element:			TIP Number:		Project	Tape:	Other	Date:	
NCDOT Conta	ct:			Contractor /	Designer:				
	Address:				Address:				
	- 1				-				
	Phone:								
	Enail:				Phone: Email:				
City/Town:	,			Countyfies):					
River Basin(s)				County?					
Wetlands with	in Project Limits?								
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Typical Cross	Section .		140						
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	ject Narrative:	Design/Future:		T cor:	Existing:			10	ar:
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To Demonstrate Attainment of the MEP Stormwater Treatment Standard

- Step 1 Establish treatment goals and document in the preliminary stormwater management plan
- Step 2- Attempt to design the project to achieve the goals
- Step 3 If necessary, identify and document site constraints impacting goal attainment
- Step 4 Design feasible BMPs
- Step 5 Document your design in the final SMP

In Conclusion

- NCDOT's statewide NPDES permit gives the Department a lot of flexibility and helps speed project delivery.
- But there's no guarantee that the flexibility NCDOT enjoys now will last. Our permit is renewed every 5 years following a detailed audit conducted by DEQ.
- So, it is incumbent upon all of us to make sure we are following these 5 steps to demonstrate MEP and ensure permit compliance.



Google: "ncdot stormwater"



NORTH CAROLINA Department of Transportation



NCDOT Hydroplaning Policy, Assessment Tool, and Mitigation Strategies

Matthew York, PE

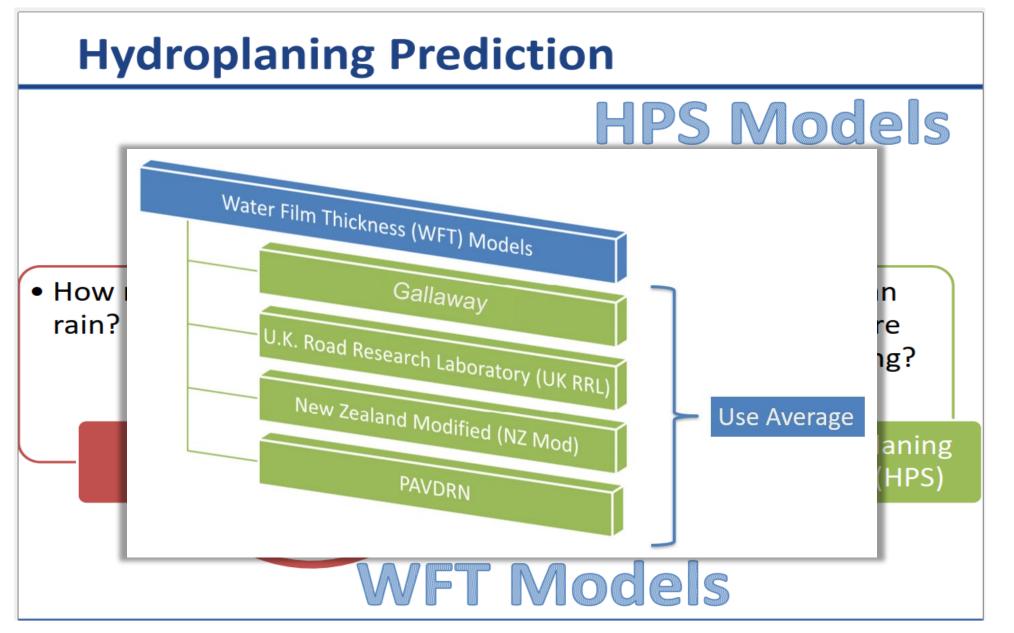
May 17, 2023

What Do We Mean by Hydroplaning?

Sheet Flow

Ponded Water





Factors that Influence Hydroplaning

Roadway and Pavement Parameters	Environmental Factors	Driver Factors	Vehicle Factors
 Surface type Rut depth Permeability of pavement surface Pavement micro- and macro-textures Cross-slope (to include locations of superelevation) Longitudinal grade (to include sag vertical curves) Pavement width Roadway curvature Depressions 	What can	 Speed Accelerating or braking Steering maneuvers we control? ck of Data	 Tire tread design Tire tread wear (tread depth) Tire pressure Vehicle type Vehicle (or axle) weight

2022 Hydraulics Guidelines

- *Preliminary* Assessment of Typical Sections
- *Final* Assessment Includes...
 - Superelevation
 - Ramps, gore areas, auxiliary lanes
- Updated Pavement Characteristics
- Modern Tire Treads and Inflation
- Mitigation Strategies



North Carolina Department of Transportation

Guidelines for Drainage Studies and Hydraulic Design

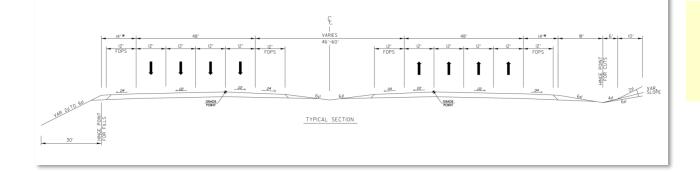
Hydraulics Unit August 8, 2022

Hydroplaning Policy - Section 4.3.2

- Required for highways with a design speed of 60 65 mph or greater and when one or both conditions occur at any point along the project:
- Tangent section with **36 ft or greater** sloped in one direction.
- Superelevated sections of 36 ft or greater, accounting for contributing directly connected impervious areas such as shoulders and gore areas.

Areas of Concern

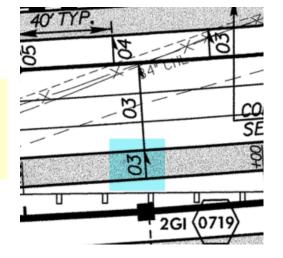
Typical Sections

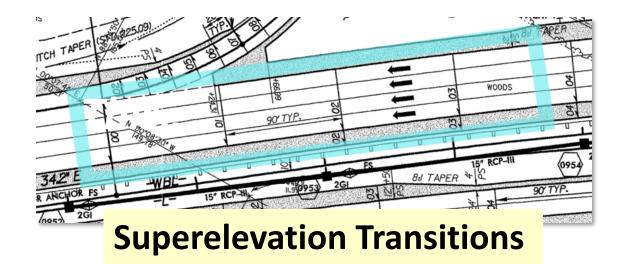


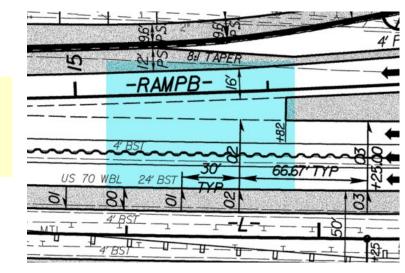
Contributing Shoulders

Ramps &

Gores







ncdot.gov

2023 Preconstruction Conference



Hydroplaning Assessment Tool

https://connect.ncdot.gov/resources/hydro/DrainageStu diesGuidelines/NCDOTHydroplaningAssessmentTool.xlsm

<u></u>			Нус	dropla	ining	Analy	/sis T	ool				
General Inputs									Date	7/26	2022	
TIP		Example					Designer		D	esigner's Na	ime	
County		Johnston				NCDO	T Divisio	on No.		Division 4		
Project Description					Short Des	pription of t	he Project					
Typical Section/Area of Concern		ection 1 - As le (tangent s 1.01	sumed 5% section) [Ex			,	Alignmen	t		L		
Assessment Type		Preliminaru	J			Station	/Milepos	t Range	Sta	a. or Mile Ma	arker	
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2023 Preconstruction Conference

 Inputs General Pavement 	TIP County Project Description Typical Section/Area of Concern Assessment Type	Example Johnston Typical Section 1 - Assumed 5% Long. Grade (tangent section) [E 1.0]	6	Designer NCDOT Division No. Description of the Project		er's Name sion 4					
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 General Pavement 	Analysis Description			Alignment	1	L					
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 Roadway 											
	Description Design Speed Cross Slope (f Width (ft.) Dense Graded Asphalt (DGAC)										
	Den Gra	aded Asphalt (DG) aded Friction Cours Pavement (PCC) n Bonded Wearing ined 70	se (OGFC)	70	70	Bhc 80					

Scenario Results

Risk Analysis Results

(Based on AVERAGE WFT and PAVDRN HPS Models)

			,	P	redicted Wat	ter Film Th	ickness (in.)							
Risk Analysis Results														
Based on AVERAGE WFT, PAVDRN I	Plane Number	1	2	3	4	5	6	7	8	9	10	11	12	
	Intensity (in/hr)	iside Shoulde	Lane 1	Lane 2	Lane 3	Lane 4	Shoulder							
F	0.1	-0.012	-0.014	-0.014	-0.009	-0.005	-0.004							
Description sid	0.25	-0.002 0.009	-0.006	-0.006	0.002	0.009	0.010							
	1	0.009	0.003	0.003	0.015	0.024	0.027							
Rainfall Intensity (in/hr)	2	0.048	0.036	0.016	0.061	0.080	0.086							
Water Film Thickness (in)	3	0.066	0.051	0.051	0.082	0.106	0.113							
	4	0.081	0.064	0.064	0.100	0.128	0.136							
Driver Speed (mph)														
Hydroplaning Speed* (mph)														
	Plane Number	1	2	3	4	5	6	7	8	9	10	11	12	
* The speed has been adjusted up +5 m	Intensity (in/hr) 0.1	iside Shoulde	Lane 1	Lane 2	Lane 3	Lane 4	Shoulder							
-	0.25	45.0 45.0	70.0	70.0	70.0 70.0	70.0 70.0	45.0 45.0							
Possible Hydroplane Conditions	0.5	45.0	64.0	64.0	64.0	64.0	45.0							
	1	45.0	62.0	62.0	62.0	62.0	45.0							
High	2	45.0	58.0	58.0	58.0	58.0	45.0							
v	3	45.0	45.0	45.0	45.0	45.0	45.0							
	4	45.0	45.0	45.0	45.0	45.0	45.0							
				D.,	a di séa di Livel	ne ni ne i	Succed (much)							
High : UDC is > 4 much Balow the Dr. ⁽¹⁾	These speeds are increased by 5M	IPH to account f	or Gunaratn	e research "	еаксеа пуа	ropianing	Speed (mph)							
High : HPS is \geq 4 mph Below the Pre	Plane Number	1	2	3	4	5	6	7	8	9	10	11	12	
Low: HPS is ≥ 2 mph Below The Pr	Intensity (in/hr)	iside Shoulde	Lane 1	Lane 2	Lane 3	Lane 4	Shoulder			-				
'	0.1	999.0	999.0	999.0	999.0	999.0	999.0							
Mitigation methods needed and prov	0.25	999.0	999.0	999.0	132.5	94.2	89.9							
· ·	0.5	93.9	122.5	122.5	82.1	73.1	71.3							
driver responsibility.	1	72.9	80.6	80.6	67.6	62.4	61.3							
None: HPS is greater than Producto	2	62.3	66.7	66.7	58.7	55.0	54.2							
None: HPS is greater than Predicte	<u>3</u> 4	57.7 54.9	61.3 58.1	61.3 58.1	54.7 52.8	52.6 52.2	52.5 52.1							
	4	04.9	08.1	1.60	02.8	0Z.Z	92.1							

Typical	Section/Area of Concern	Rollove	r Transition	[Ex 3.0]				Sui	oer	ele	eva	tio	n
	Assessment Type		Final						1				-
A	nalysis Description	Supere	elevation Tra	insition		Transition							
Sup	erelevation Transition	# of lanes	3	Factor	1.5								
		Faile with	Donco Grad	od Acobalt									
	Risk Analysis R	esults											
Pavem	Based on AVER	RAGEW	/FT, PA	VDRN	HPS M	odel, a	nd a w	orst-ca	ise sce	enario	rainfall	intensi	ity (in/hr)
Longitu	Max WF1	r Condi	tion wi	thin the	e SE Ro	ollover							
	Rainfall Intensit	ty (in/hr)			2.0							
Surface	Water Film Thio	kness	(in)		0	.106							
	Driver Speed (n	nph)			ļ	58.0							
	Hydroplaning S	speed*	(mph)			52.6							
Descrip	* The speed has	been a	djusted	up +5 r	mph to a	account	for Mo	dern Ti	res.				
Design	Speed (mph)	45	70	70	70	70	45						
Cross S	lope (ft/ft)	-0.04	-0.02	0.02	0.02	0.02	0.04						
Width (f	t.)	12	12	12	12	12	14						

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2023 Preconstruction Conference

Hydraulics Designer:	Designer's Nam	e											\sim					
Reviewer:				Final Hydroplaning Assessment									000	- 1	15ª			
Date:	7/26/2022																	
Project TIP #:	Example																	
Area of Concern	Analysis Description	Alignment	Station	Direction	Lane	Design Speed (mph)	Rainfall Intensity (I, in/hr)	Long. Slope (%)	Cross Slope (ft/ft)	Pavement Width (ft)	Pavement Surface Material	Hydroplaning Average WFT (in)	PAVDRN Hydroplaning Speed (mph)	Predicted Driver Speed (mph)	Analysis Notes	User-Defined Pavement Notes		
Typical Section 1 - Max. Long Grade (tangent section) [Ex 2.0]	Greater than 36 ft. of Impervious Pavement	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	3.3	0.02	36.0	DGAC	0.07	56.1	58.0	0.02 ft/ft cross slope on the exterior produces more potential for hydroplaning			
Typical Section 1 - Max. Long Grade (tangent section) [Ex 2.1]	Mtigation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	3.3	0.03	36.0	DGAC	0.07	57.6	58.0	No Hydroplaning potential w/ 0.03 ft/ft Exterior Lane cross slope			
Typical Section 1 - Max. Long Grade (tangent section) [Ex 2.2]	Mitgation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	3.3	0.03	36.0	PCC	0.06	60.1	58.0	Concrete Pavement (PCC) shows no potential for Hydroplaning			
Typical Section 1 - Max. Long Grade (tangent section) [Ex 2.3]	Mitgation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	3.3	0.03	36.0	UTBWC	0.05	60.9	58.0	Ultra-Thin Bonded Wearing Course Surface shows no potential for Hydroplaning			
Typical Section 1 - Max. Long Grade (tangent section) [Ex 2.4]	Migation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	3.3	0.03	36.0	OGFC	0.04	63.8	58.0	Open Graded Friction Course Surface shows no potential for Hydroplaning			
Rollover Transition [Ex 3.0]	Superelevation Transition	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	2.5	0.02	36.0	DGAC	0.11	52.6	58.0	Fails with Dense Graded Asphalt (DGAC)			
Rollover Transition [Ex 3.1]	Superelevation Mitigation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	2.5	0.02	36.0	PCC	0.09	54.6	58.0	Fails with Concrete Pavement (PCC)			
Rollover Transition [Ex 3.2]	Superelevation Mitigation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	2.5	0.02	36.0	UTBWC	0.09	55.5	58.0	Shows a high potential for hydroplaning conditions			
Rollover Transition [Ex 3.3]	Superelevation Mitigation	L	Sta. or Mile Marker	Northbound	Lane 4	70.0	2.0	2.5	0.02	36.0	OGFC	0.07	60.3	58.0	Open Graded Friction Course Surface Passes			
Initial Uniform Cross-slope After Rollover Transition [Ex 4.0]	Greater than 36 ft. of Impervious Pavement	L	Sta. or Mile Marker	Northbound	Lane 1	70.0	2.0	2.5	-0.02	24.0	DGAC	0.09	54.2	58.0	Fails when using Dense Graded Asphalt (DGAC)			
Initial Uniform Cross-slope After Rollover Transition [Ex 4.1]	Mitigation	L	Sta. or Mile Marker	Northbound	Lane 1	70.0	2.0	2.5	-0.02	24.0	PCC	0.07	56.1	58.0	Concrete Pavement (PCC) Shows more potential for hydroplaning conditions			
Initial Uniform Cross-slope After Rollover Transition [Ex 4.2]	Mitigation	L	Sta. or Mile Marker	Northbound	Lane 1	70.0	2.0	2.5	-0.02	24.0	UTBWC	0.07	56.7	58.0	Ultra-Thin Bonded Wearing Course Shows low potential for hydroplaning conditions			
Initial Uniform Cross-slope After Rollover Transition [Ex 4.3]	Mitigation	L	Sta. or Mile Marker	Northbound	Lane 1	70.0	2.0	2.5	-0.02	24.0	OGFC	0.06	60.8	58.0	Open Graded Friction Course Surface shows no potential for Hydroplaning			

Mitigation Selection Guide

	Hydroplaning Mitigation Selection Guide																
	U_	РА	VEMENT	MITIGATIC	N				GEOME		SIGNAGE						
	Pav	ement Over	lays	Surface Treatments				Modifying Roadway Typical		Intercepting Pavement Runoff		Managing Roadway Geometry			Signage Strategies		
Select mitigation topic for more information>	Open Graded Friction Course ¹	Ultra Thin Bonded Wearing Course	High Surface Friction Treatment ³	Diamond Grooving ⁴	Diamond Grinding	Shotblasting	Slope Shoulder Away	Moving the Crown Point	Gore Valley Gutters	Slotted or Trench Drain	Flatten Longitudinal Slope	Increase Cross Slopes	Adjust SE Transitions	Static Signs	Static Signs with Emphasis	Dynamic Signs	
Applicable Project Type (New Pavement ² , Widening, Maintenance)	All	All	Widening or Maintenance	Widening or Maintenance	All	Maintenance	All	All	Widening or Maintenance	Widening or Maintenance	New Pavement	All	All	All	All	New Pavement or Widenings	
Spatial Exent ⁵	Global	Global	Local	Both	Both	Both	Global	Global	Local	Local	Global	Global	Both	Both	Both	Both	
Construction Costs	\$\$	\$\$	\$\$\$	\$\$\$	\$\$\$	\$\$	\$	\$	\$	\$\$	\$	\$	\$\$	\$	\$	\$\$	
Maintenance Effort	medium	medium	high	medium	medium	high	low	low	low	medium	low	low	low	low	low	medium	
Service Life	8-10 years	9-11 years	8-12 years	15 years	15 years	2-5 years	N/A	N/A	N/A	N/A	N/A	N/A	N/A	10 years	10 years	15-20 years	
Suitable for the Following Existing Pavement Surfaces ⁶	DGAC Concrete UTBWC	DGAC Concrete	DGAC Concrete	Concrete DGAC ⁴	Concrete	DGAC Concrete OGFC	All Pavement Surfaces	All Pavement Surfaces	All Pavement Surfaces	All Pavement Surfaces	All Pavement Surfaces	All Pavement Surfaces	All Pavement Surfaces	Applicable for all pavement surfaces inc temporary construction conditions		7	
Hydroplaning Benefit ★ to ★★★	***	**1	***	★★★ (transverse grooving)	**	**	*	***	***	***	*	**	***		ess is unknown a er research is ne		

Notes:

General Note: Mitigation strategies can be combined for greater hydroplaning potential reduction. Example: geometry, pavement type, or surface treatment.

(1) Open Graded Friction Course is not recommended for regions prone to frequent ice/snow events or longitudinal slopes steeper than 5%. (Divisions 11, 13 and 14)

(2) New pavement consists of new and/or reconstructed pavement.

(3) High Friction Surface Treatment is only applicable for DGAC or Concrete pavement and treatment is vulnerable to maintenance issues in Divisions where sand is used in ice/snow conditions.

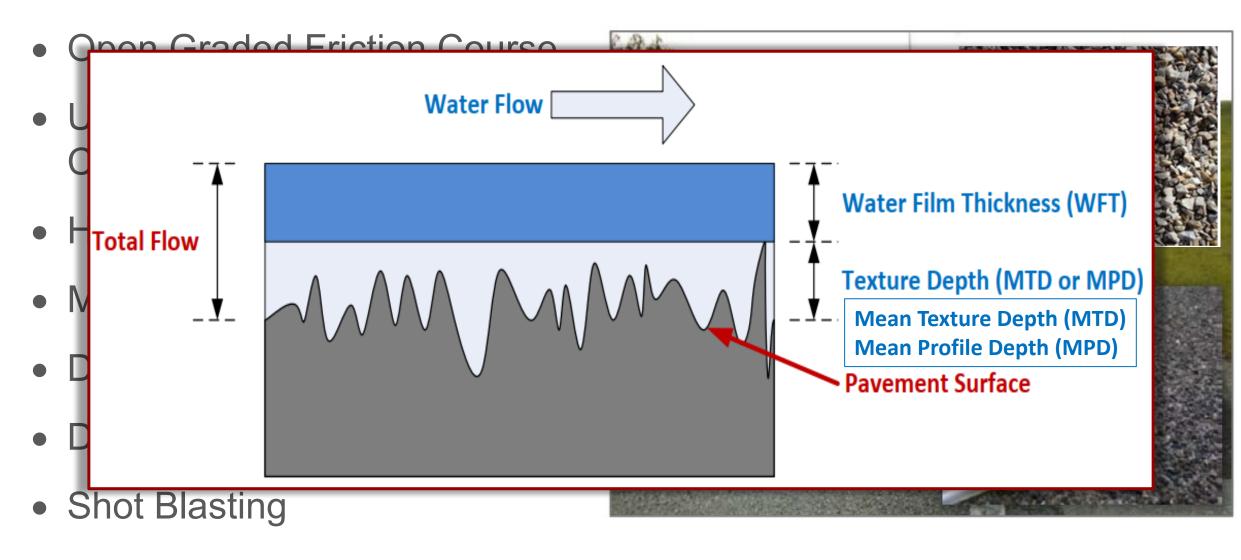
(4) Diamond grooving is typically reserved for bridge decks (see NCDOT Specification 420). DGAC grooving can be used for short segments, typically curves, as a spot treatment.

(5) Global treatments are applicable to the entire project limits; Local treatments are considered 'spot treatments' and used in smaller applications.

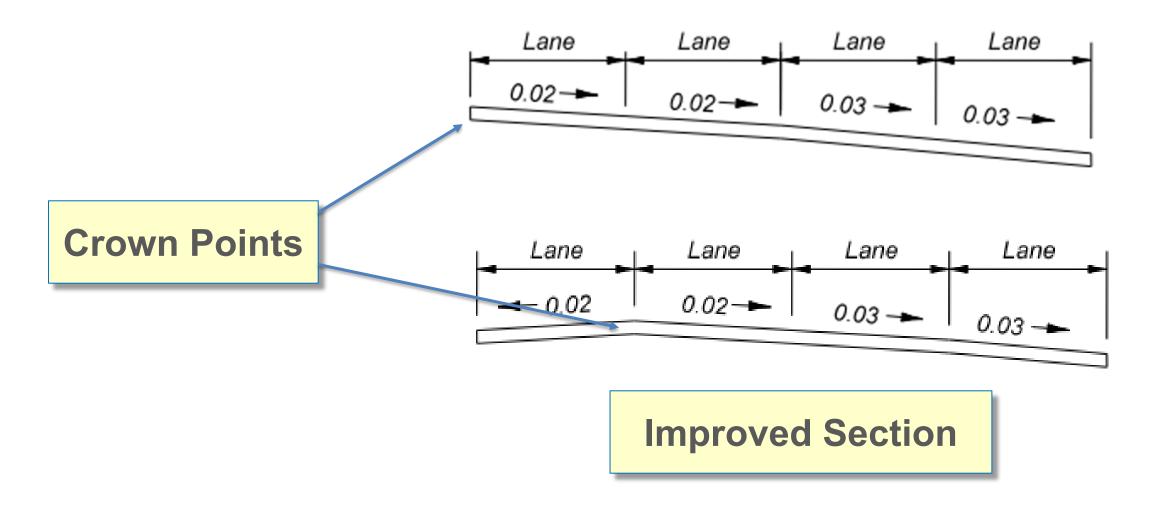
(6) If hydroplaning potential occurs in OGFC areas, consider geometric solutions.

(7) Variable message boards can be used during construction to warn of temporary hydroplaning concerns.

Pavement Surface Improvements

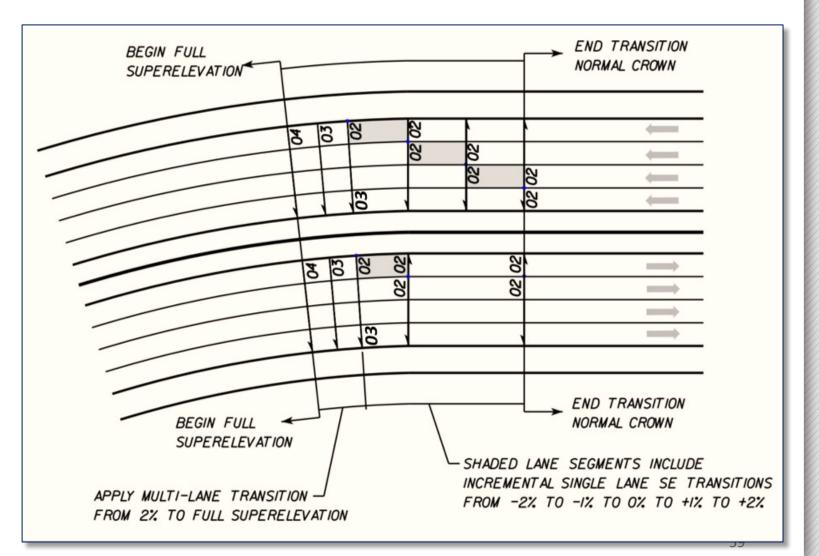


Moving the Crown Point



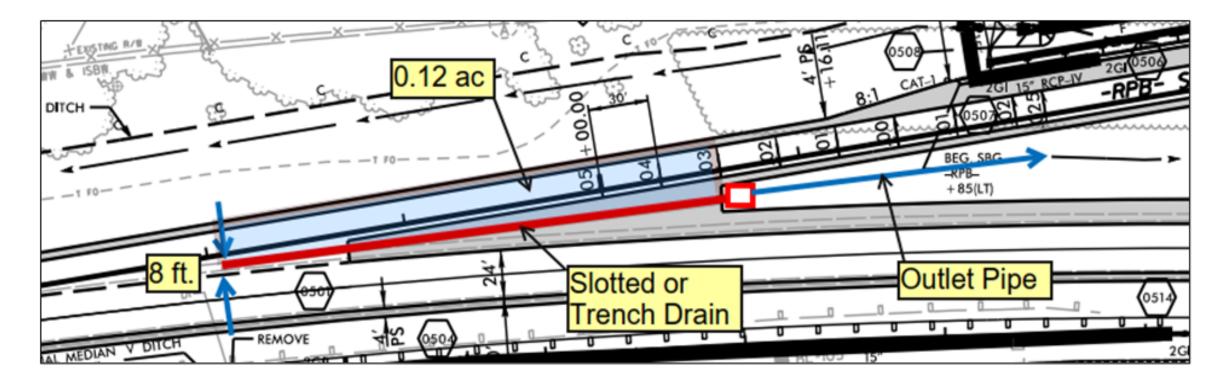
Adjusting Superelevation Transitions

- Transition One
 Lane at a Time
- Results in Longer
 Transitions
- Not always feasible



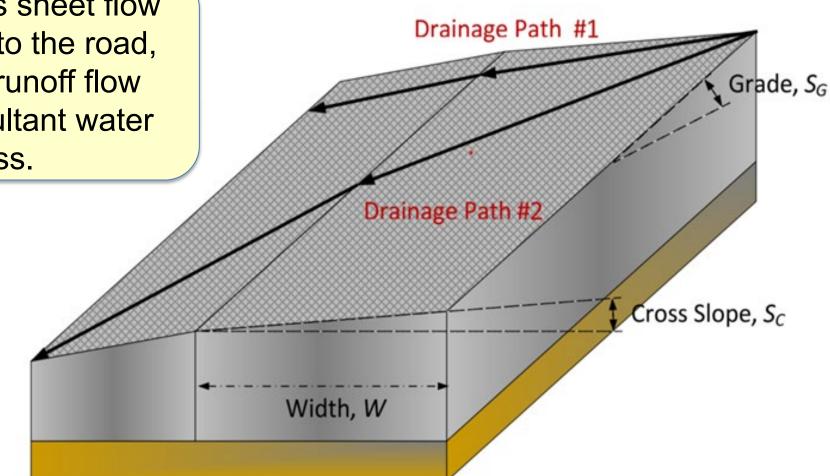
Intercepting Pavement Runoff

- Paved gore gutters where gore slope is favorable
- Slotted Drains / Trench Drains up to about 0.3% adverse gore slope



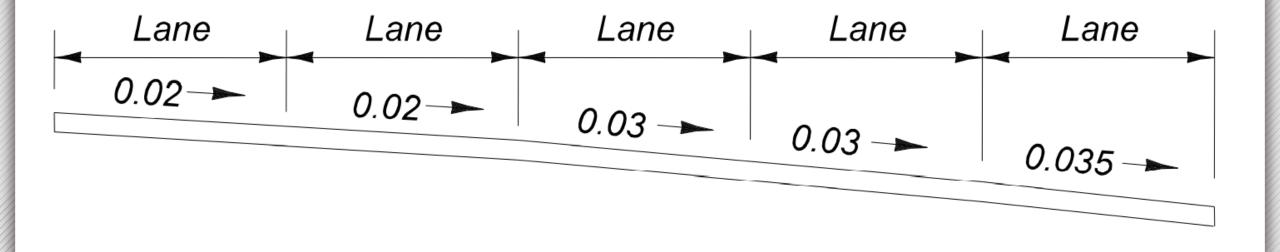
Flatter Longitudinal Slopes

Flatter grades makes sheet flow more perpendicular to the road, which reduces the runoff flow path length and resultant water film thickness.



Increase Cross Slopes

Increase the cross-slope steepness to provide faster and more efficient removal of water from the pavement. AASHTO recommends breaking the cross slope every two lanes and prohibits cross slopes greater than 3.5% on high-speed facilities.



Static and Active Warning Signs



- Static Signs
- Static Signs with Emphasis
- Dynamic Signs



What's Next for Hydroplaning?





Hydroplaning Tool 2.0

- Improved automation
- Designer flexibility

Future Research

- NC Driver Behavior Study
- MPD Data from Pavement Studies

LUNCH & LEARN COMING SOON!

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